

Dewatering

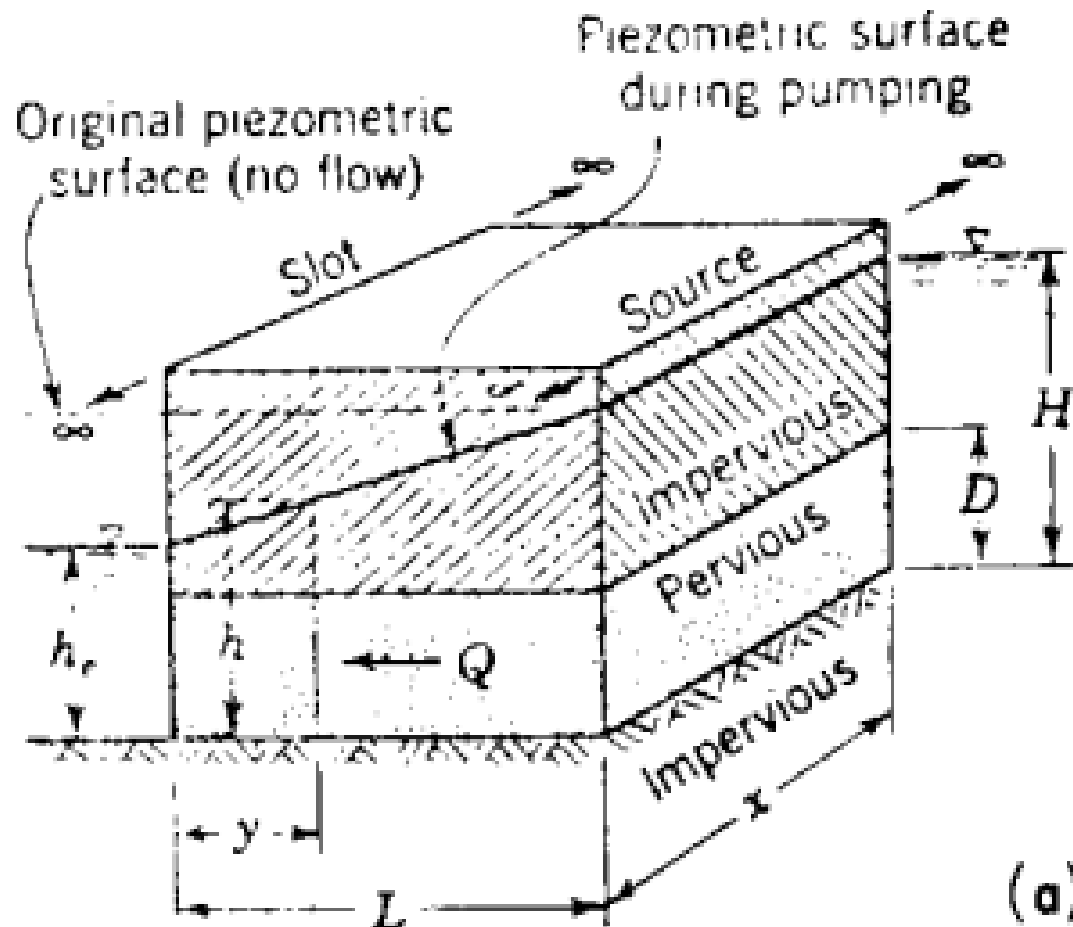
GRADUATION PROJECT

GEOTECHNICAL ENGINEERING

Contents

- Flow into Slots
- Flow into Wells
- Dealing with a Line Source
- Dealing with side support systems
- Neighboring Structures
- Permeability Estimation
- Design of Plug
- Detailing

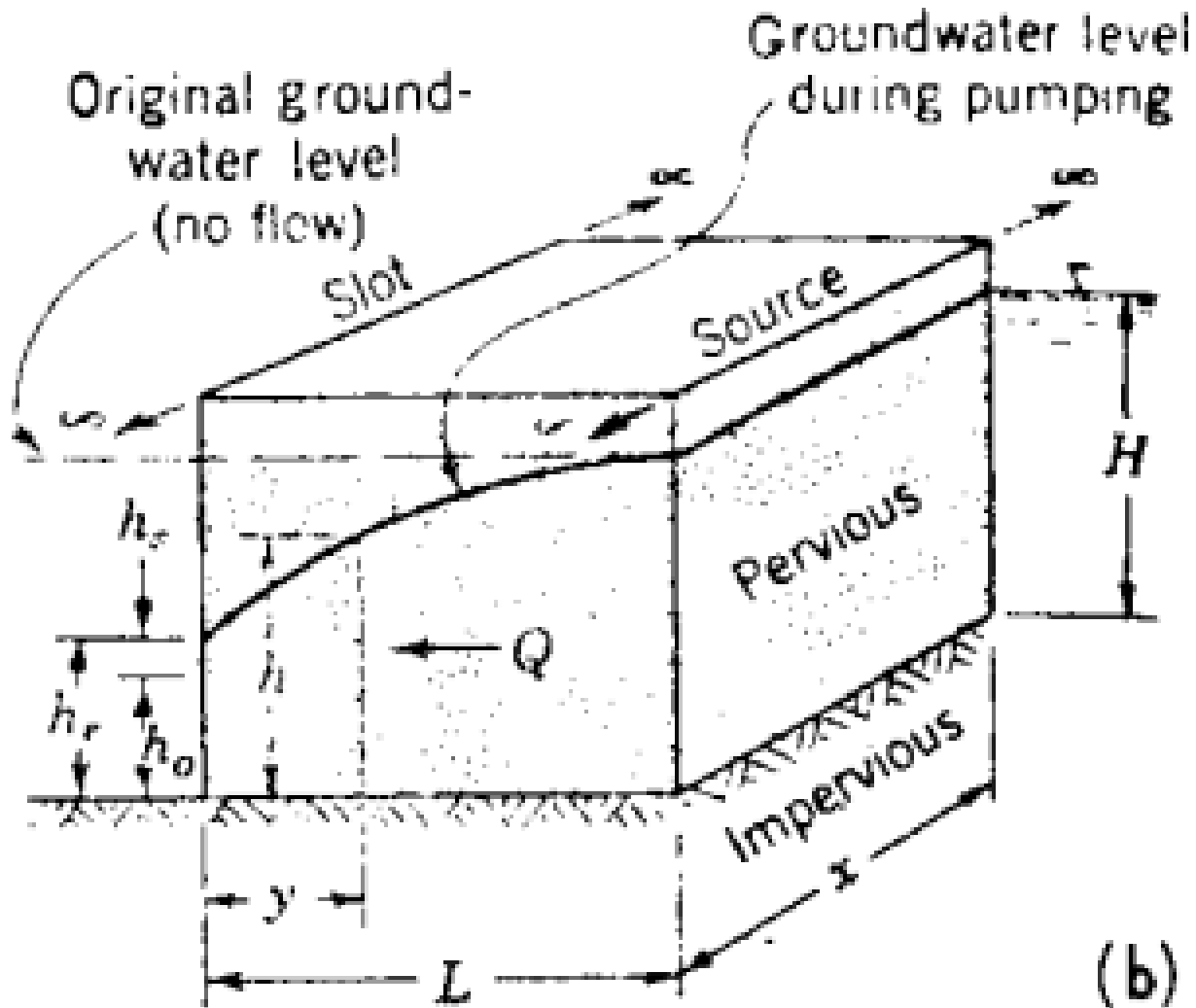
Flow and drawdown to Slots



$$Q = \frac{KD}{L} (H - h_e)$$

per meter length

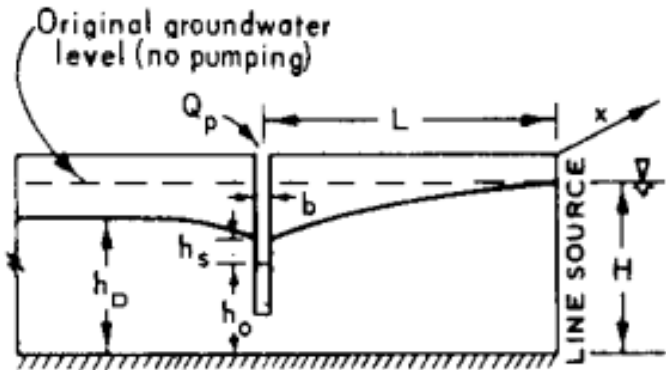
Flow and drawdown to Slots



$$Q = \frac{K}{2L} (H^2 - h_0^2)$$

per meter length

Partial Penetrating Slots



h_s IS OBTAINED FROM FIG. 4-2:

(c)

FLOW

$$Q_p = \left(0.73 + 0.27 \frac{H - h_o}{H} \right) \frac{kx}{2L} (H^2 - h_o^2) \quad (3)$$

WHERE $L \geq 3H$

GRAVITY FLOW

MAX RESIDUAL HEAD DOWNSTREAM OF SLOT

$$h_D = h_o \left[\frac{1.48}{L} (H - h_o) + 1 \right] \quad (4)$$

$$R \approx C (H - h_w) \sqrt{k} \quad (1)$$

Equivalence to well point system

where $L =$ distance to line source

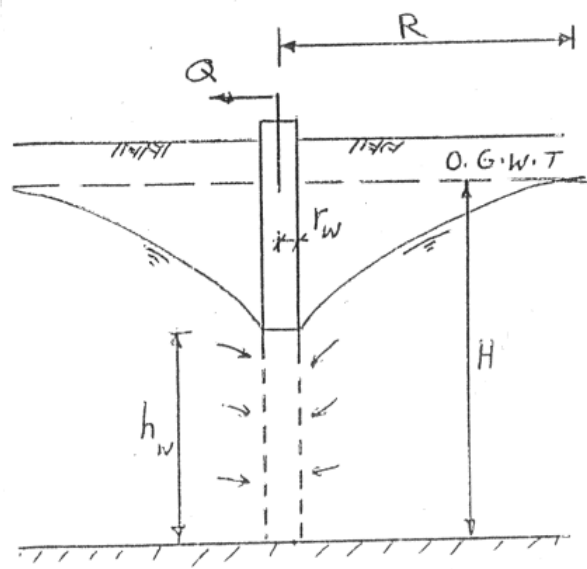
OR $L \approx 150 (H - h_o) \sqrt{k} \dots \dots \dots$ (k in cm/sec)

WHERE $R, H,$ AND h_w ARE DEFINED PREVIOUSLY AND EXPRESSED IN FEET. COEFFICIENT OF PERMEABILITY, k , IS EXPRESSED IN 10^{-4} CM/SEC.

AND $C \approx 3$ FOR ARTESIAN AND GRAVITY FLOWS TO A WELL.

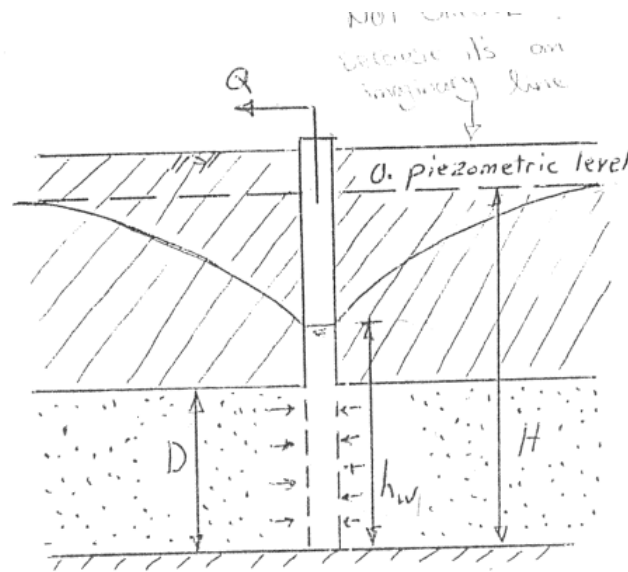
$C \approx 1.5$ TO 2.0 FOR A SINGLE LINE OF WELLPOINTS.

Flow to Wells



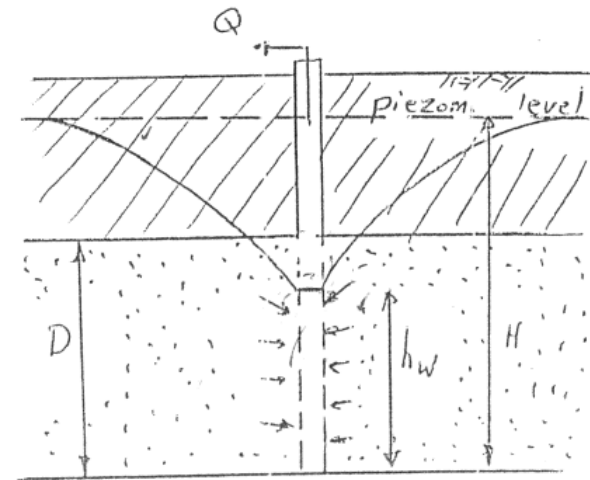
unconfined
Gravity Flow

$$Q = \frac{\pi K (H^2 - h_w^2)}{\ln R/r_w}$$



confined aquifer
Artesian Flow

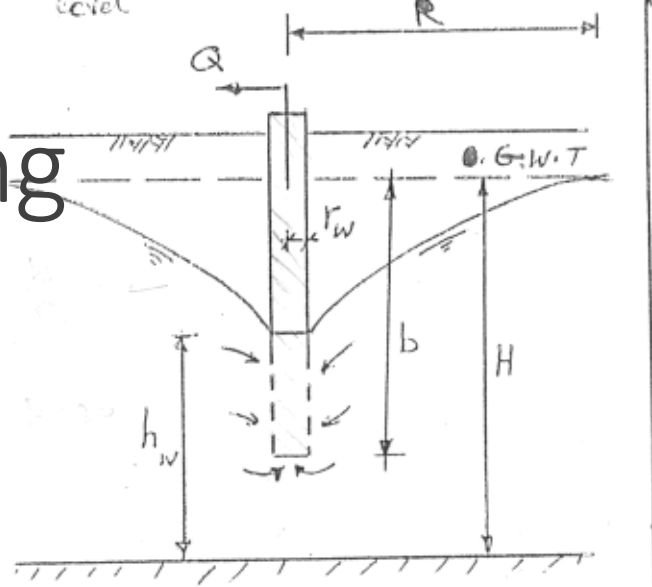
$$Q = \frac{2\pi K D (H - h_w)}{\ln (R/r_w)}$$



semiconfined aquifer
Mixed Flow

$$Q = \frac{\pi K (2DH - D^2 - h_w^2)}{\ln (R/r_w)}$$

Partially Penetrating Wells

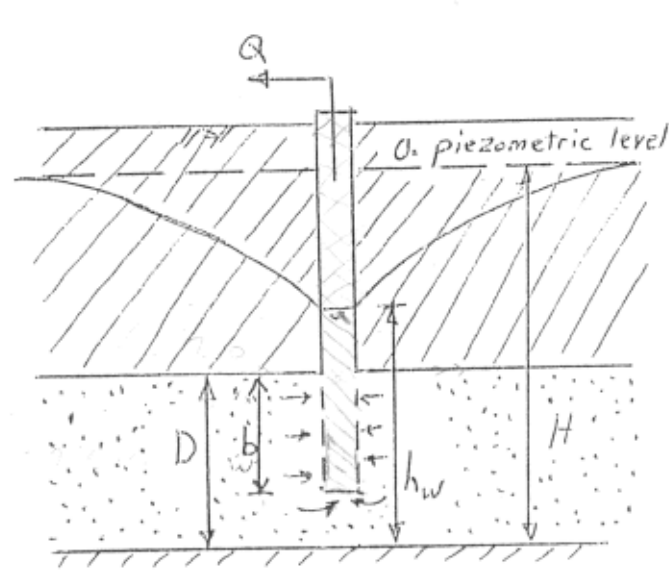


Gravity Flow

$$Q = \frac{\pi k \beta (H^2 - h_w^2)}{\ln R/r_w} \cdot G$$

$$\beta = \frac{b}{H}$$

↓ penetration ratio

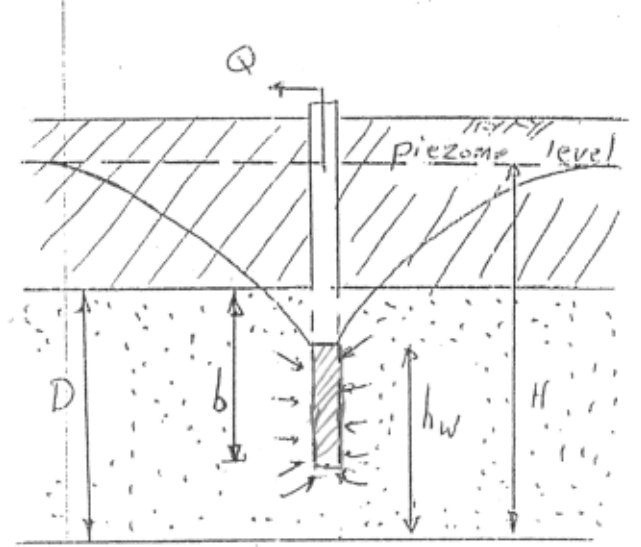


Artesian Flow

$$Q = \frac{2\pi k \beta D (H - h_w)}{\ln (R/r_w)} \cdot G$$

$$\beta = \frac{b}{D}$$

↓ penetration ratio



Mixed Flow

$$Q = \frac{\beta \pi k (2DH - D^2 - h_w^2)}{\ln (R/r_w)} \cdot G$$

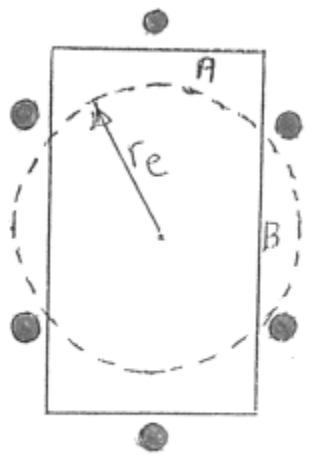
$$\beta = \frac{b}{D}$$

↓ penetration ratio

$$G = 1 + 7 \sqrt{\frac{r_w}{2b}} \cdot \cos\left(\frac{\pi \beta b}{2}\right) > 1$$

↳ consider water enters from the bottom of the well.

Design Process for Multiple Wells



$$r_e = \sqrt{\frac{A \times B}{\pi}}$$

$$R \approx 300(H - h_w) \sqrt{\kappa}$$

$$H^2 - h_p^2 = \frac{1}{\pi k} \sum_{i=1}^n q_{wi} \ln \frac{R}{r_i} \dots \dots \dots \text{(Gravity) ...}$$

$$H - h_p = \frac{1}{2\pi k D} \sum_{i=1}^n q_{wi} \ln \frac{R}{r_i} \dots \dots \dots \text{(Artesian) ..}$$

$$2DH - D^2 - h_p^2 = \frac{1}{\pi k} \sum_{i=1}^n q_{wi} \ln \frac{R}{r_i} \dots \dots \dots \text{(Mixed)}$$

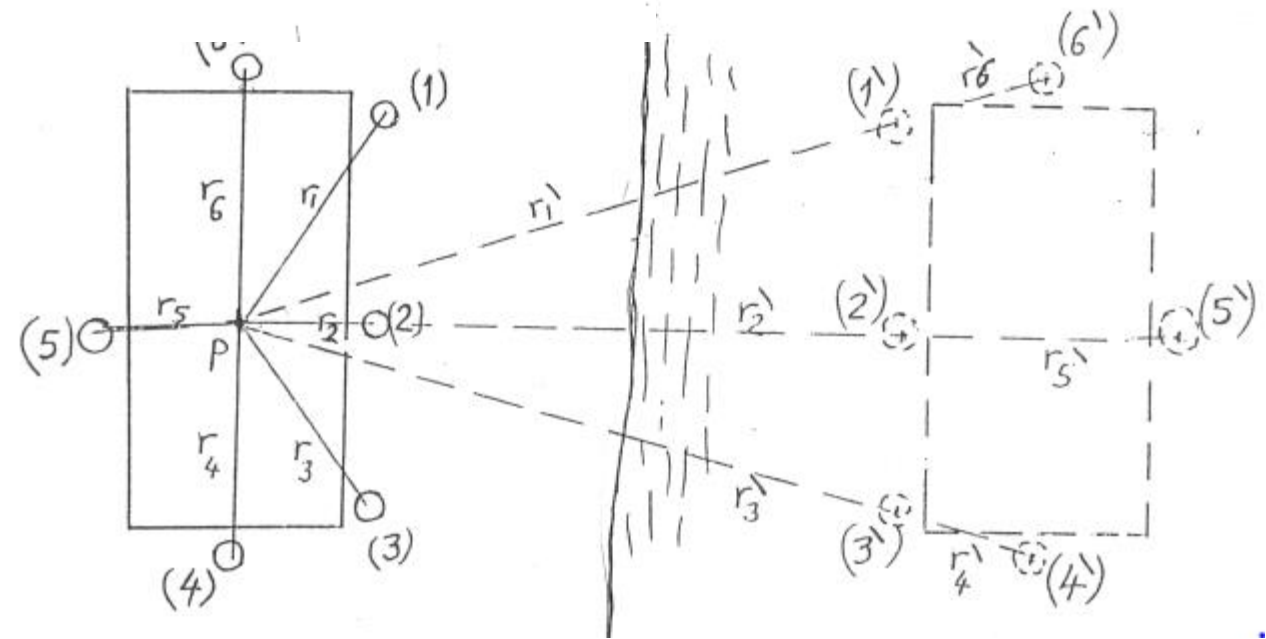
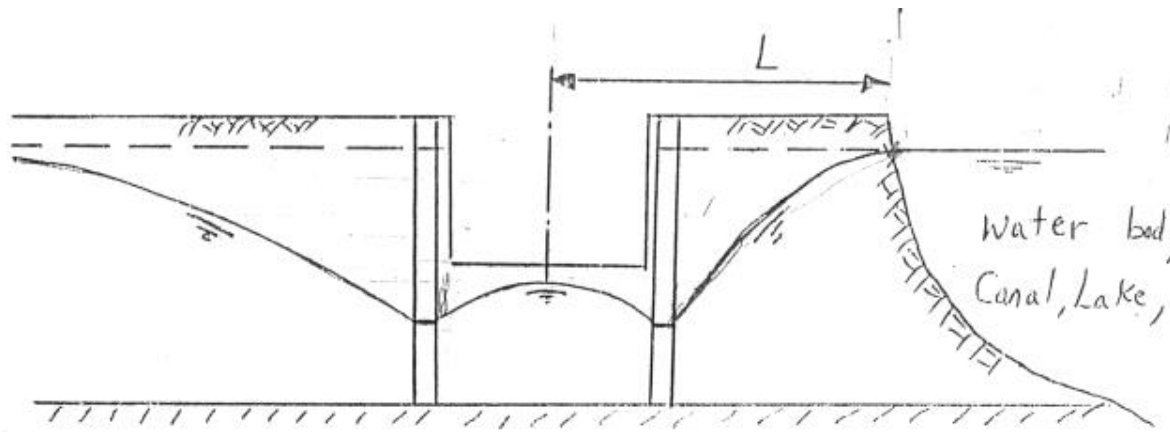
$$Q_T = \frac{\pi k (H^2 - h_w^2)}{\ln(R/r_e)} \dots \dots \dots \text{(Gravity flow)}$$

$$= \frac{2\pi D k (H - h_w)}{\ln(R/r_e)} \dots \dots \dots \text{(Artesian flow)}$$

$$= \frac{\pi k (2DH - D^2 - h_w^2)}{\ln(R/r_e)} \dots \dots \dots \text{(Mixed flow) S}$$

Dealing with a nearby line source

There exists a water body (Sea - River, Canal - lake) near the dewatered area at distance $L < R_1$



Modified Design Process

$$\begin{aligned}
 Q_T &= \frac{\pi k (H^2 - h_w^2)}{\ln(2L/r_e)} \dots\dots\dots \text{(Gravity)} \dots\dots\dots \\
 &= \frac{2\pi Dk(H - h_w)}{\ln(2L/r_e)} \dots\dots\dots \text{(Artesian)} \dots\dots\dots \\
 &= \frac{\pi k (2DH - D^2 - h_w^2)}{\ln(2L/r_e)} \dots\dots\dots \text{(Mixed)} \dots\dots\dots
 \end{aligned}$$

$$H^2 - h_p^2 = \frac{1}{\pi k} \sum_{i=1}^n q_{wi} \ln \frac{r_i'}{r_i} \dots\dots\dots \text{(Gravity)} \dots\dots\dots$$

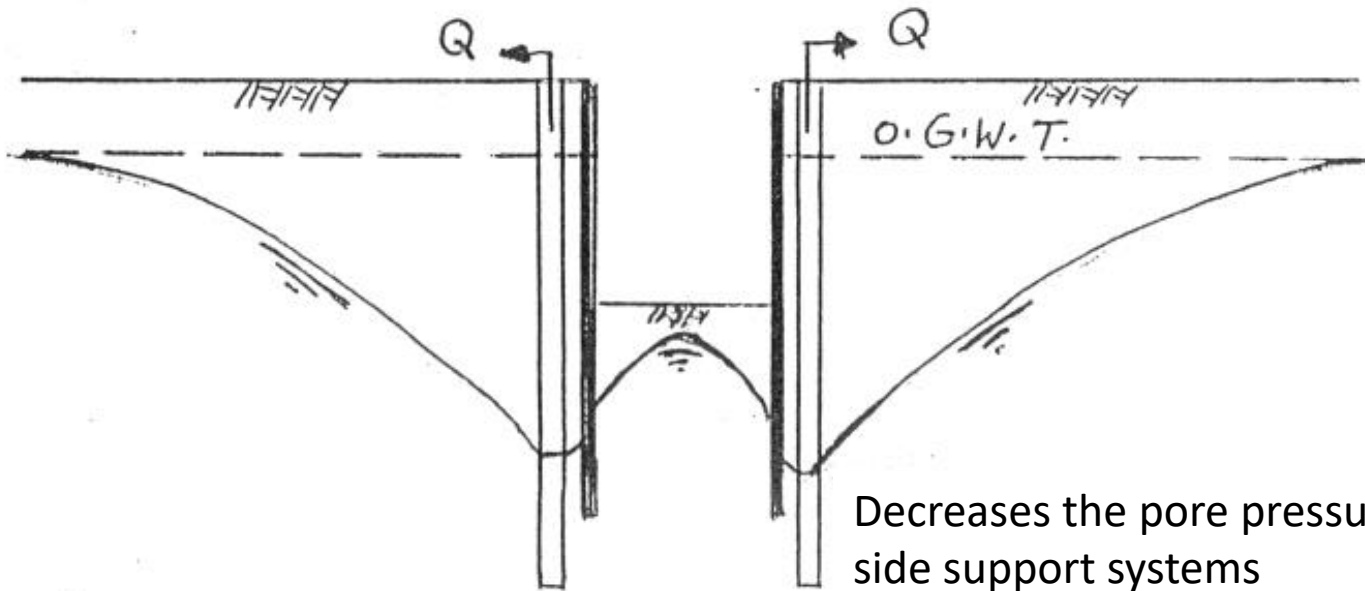
$$H - h_p = \frac{1}{2\pi kD} \sum_{i=1}^n q_{wi} \ln \frac{r_i'}{r_i} \dots\dots\dots \text{(Artesian)} \dots\dots\dots$$

$$2DH - D^2 - h_p^2 = \frac{1}{\pi k} \sum_{i=1}^n q_{wi} \ln \frac{r_i'}{r_i} \dots\dots\dots \text{(Mixed)} \dots\dots\dots$$

where r_i = distance from point "P" to the i^{th} real well
 r_i' = distance from point "P" to the i^{th} image well

Dealing with Side support systems

Outside the excavation

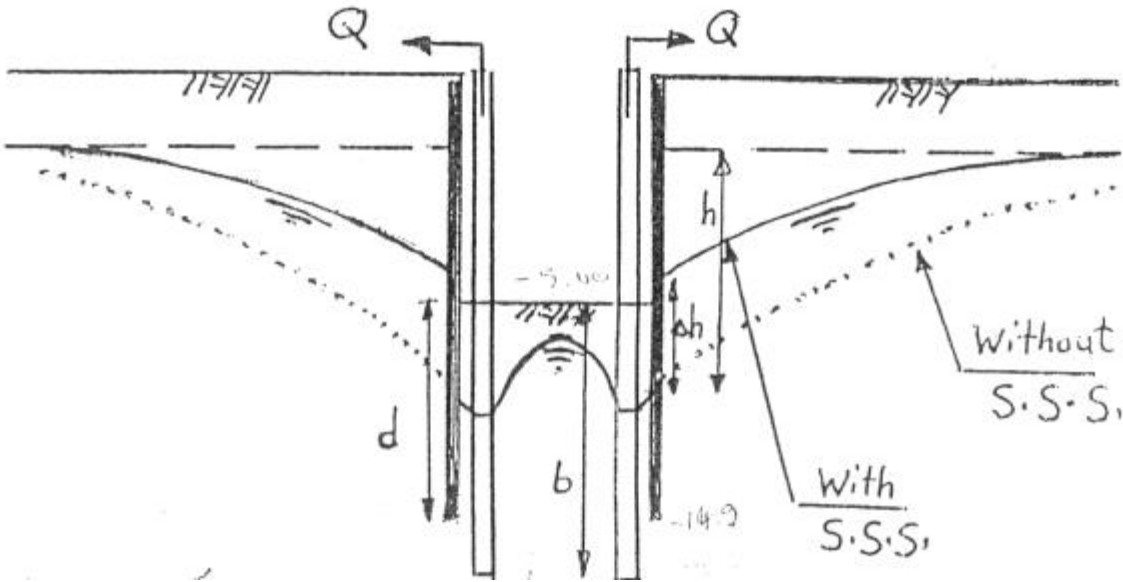


Issues

- Intrusion on the neighbors
- Affecting neighboring structures

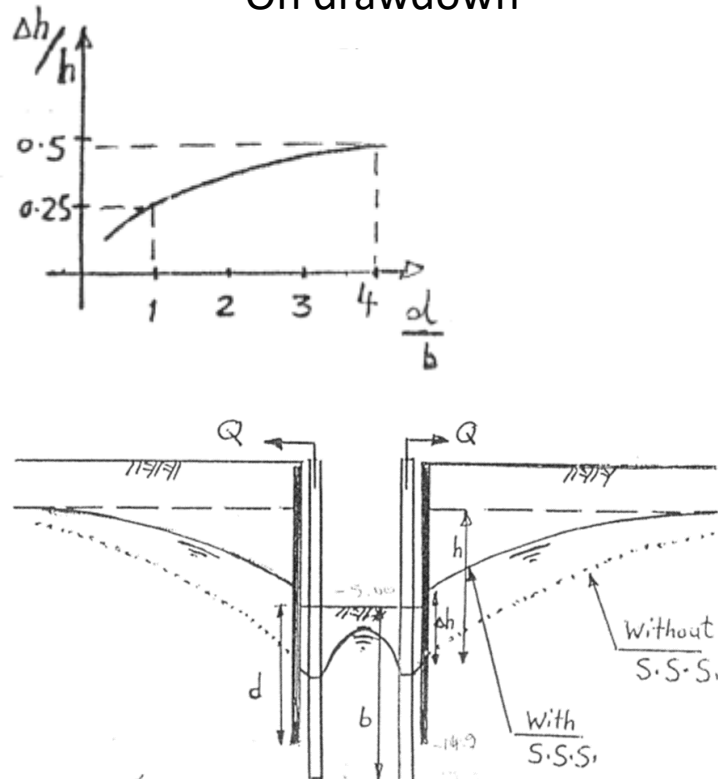
Dealing with Side support systems

Inside the excavation

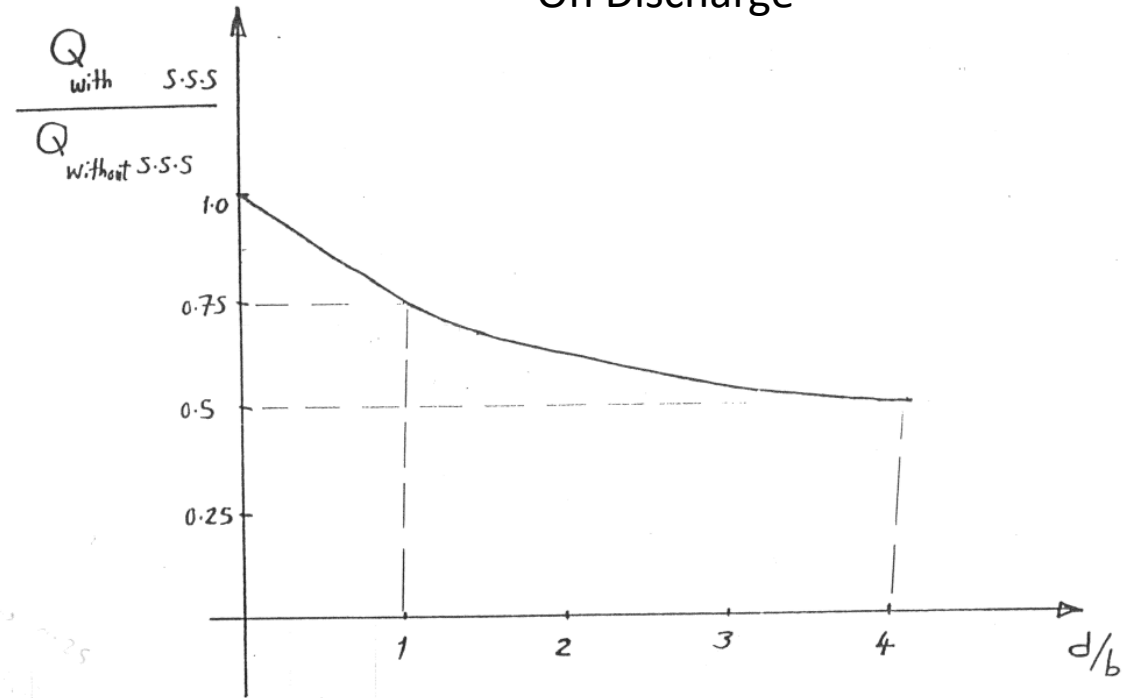


Effect of Side Support System

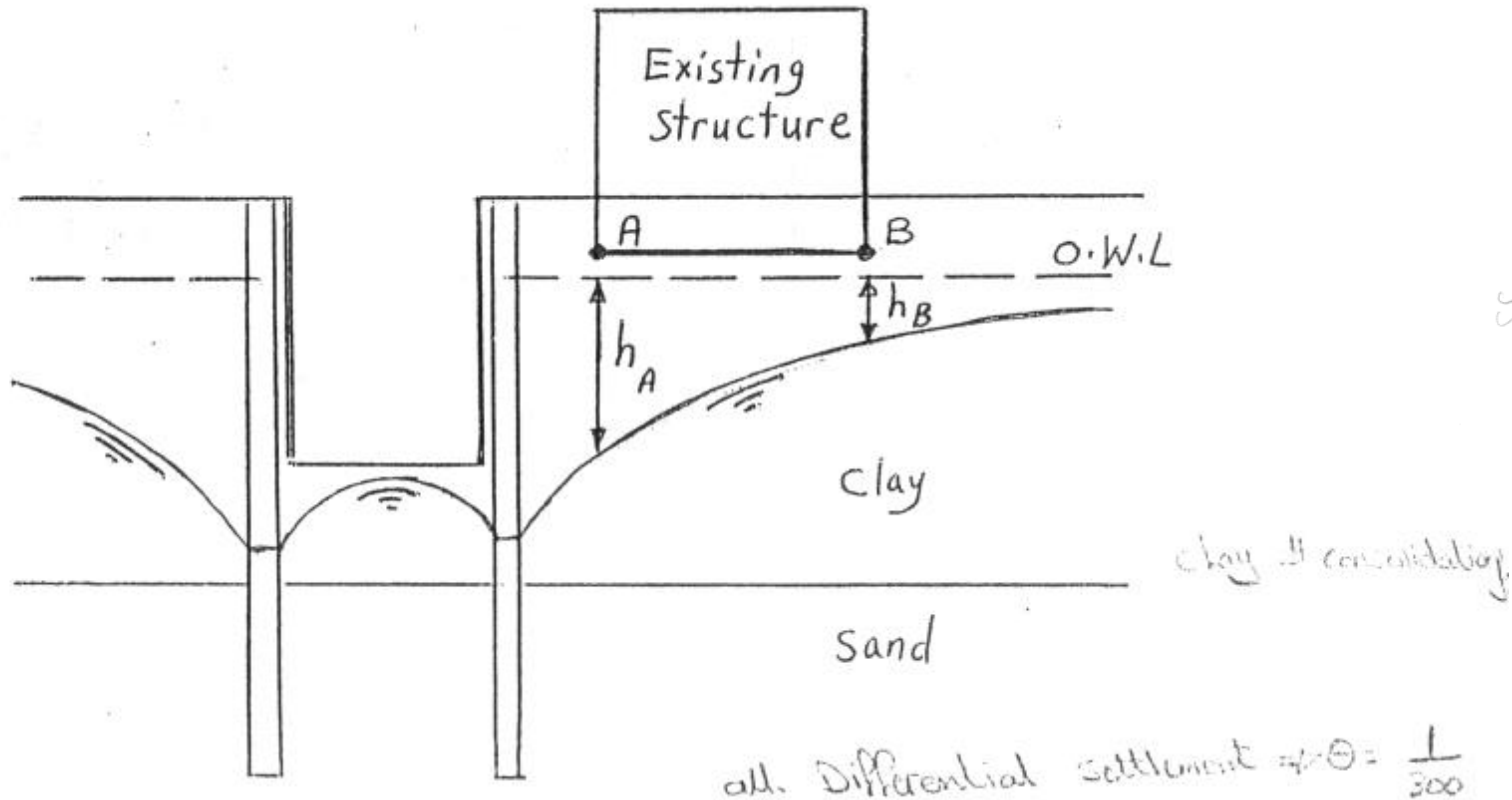
On drawdown



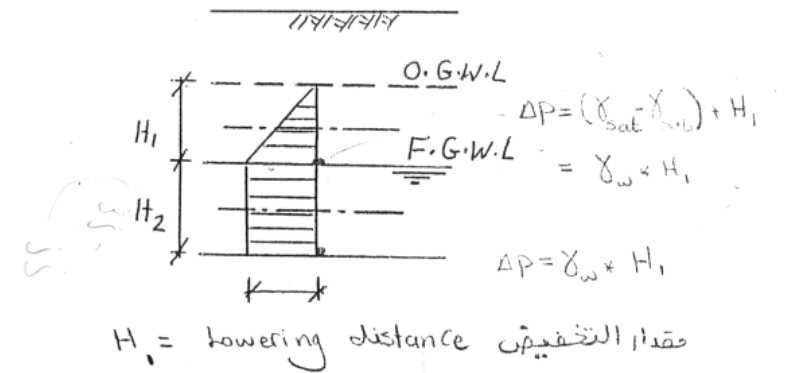
On Discharge



Neighboring Structures



all. Differential settlement $\Rightarrow \theta = \frac{L}{300}$



$$\underline{\underline{\Delta P = \gamma_w \cdot H_1}}$$

$$S_1 = m_{v1} \cdot \left(\frac{1}{2}\right) \Delta P \cdot H_1$$

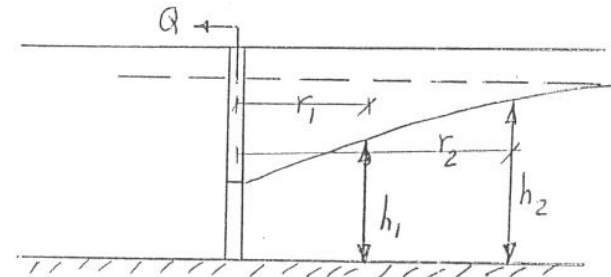
$$S_2 = m_{v2} \cdot \Delta P \cdot H_2$$

$$S = S_1 + S_2$$

Permeability Estimation

Field Methods

$$Q = \frac{\pi K (h_2^2 - h_1^2)}{ln r_2/r_1} \rightarrow K,$$



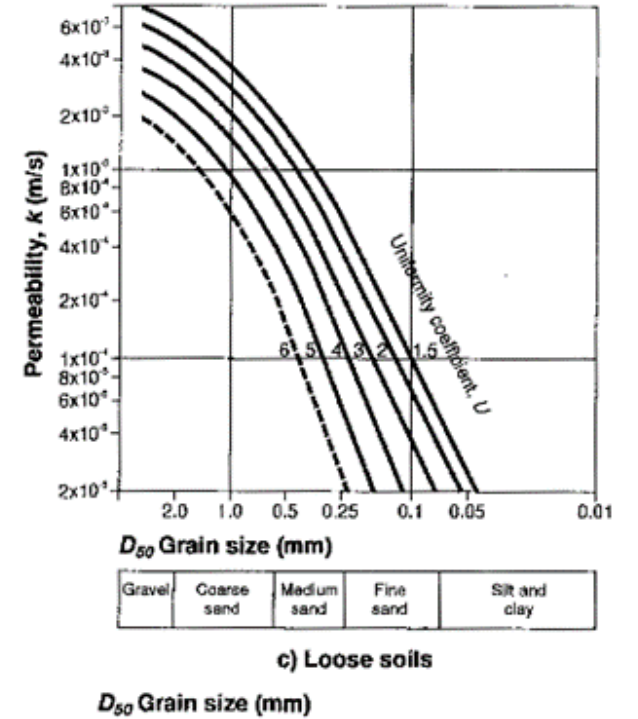
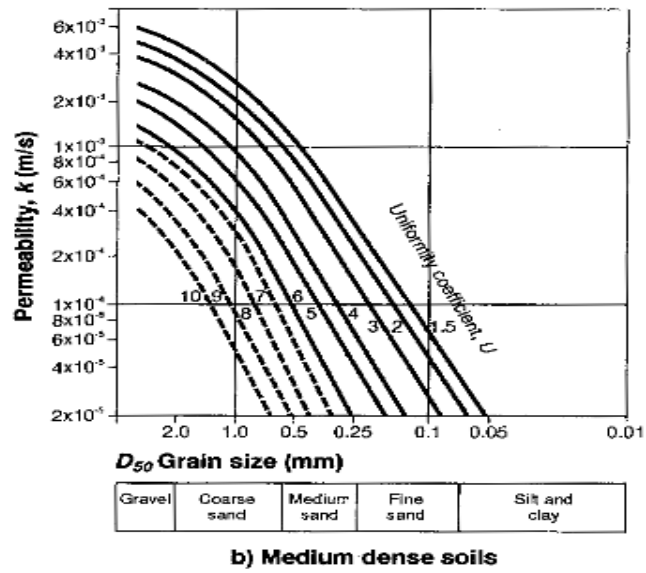
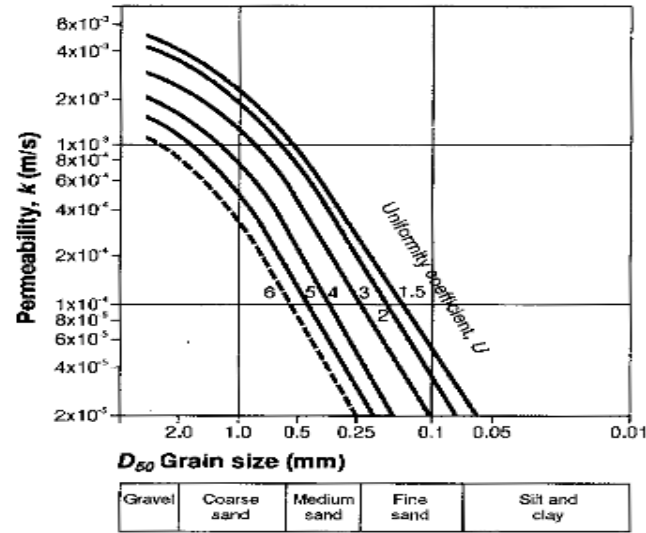
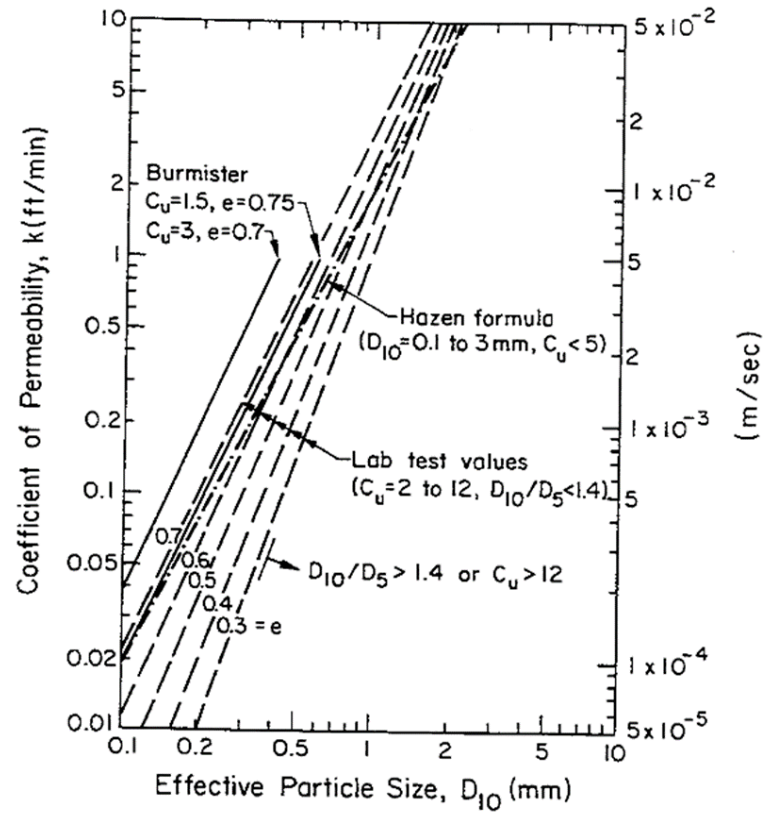
Laboratory Methods

Constant head (sand)
 Falling head (clay)

Correlations

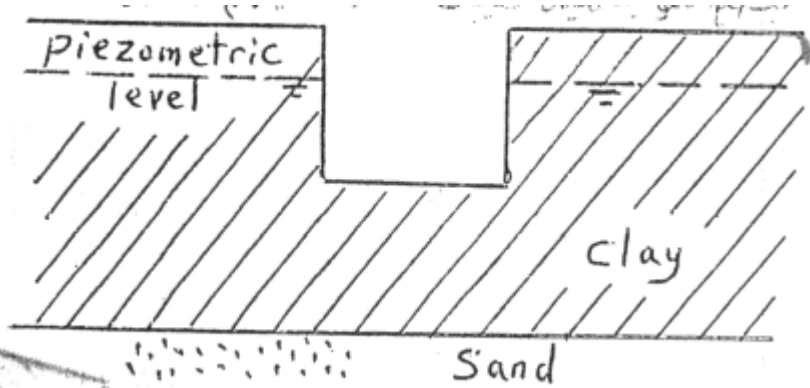
Hazen Formula	$K \approx 100 d_{10}^2$
K (cm/sec)	d_{10} (cm)

Correlations

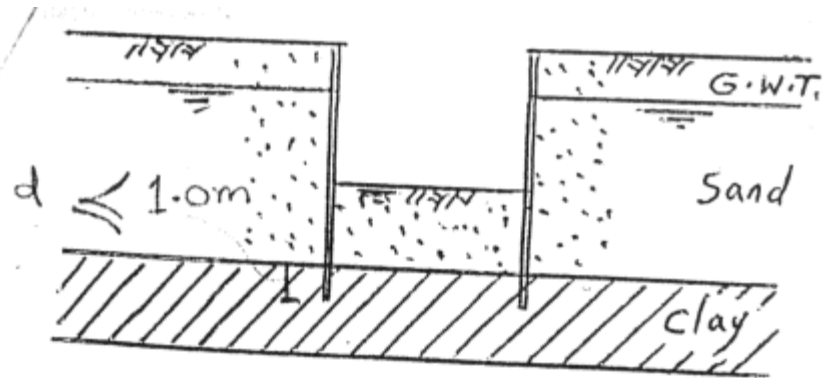
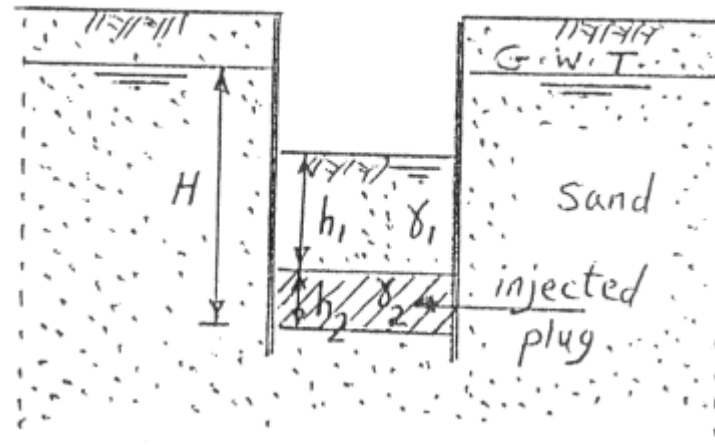


Cut off Systems

Natural Plugs



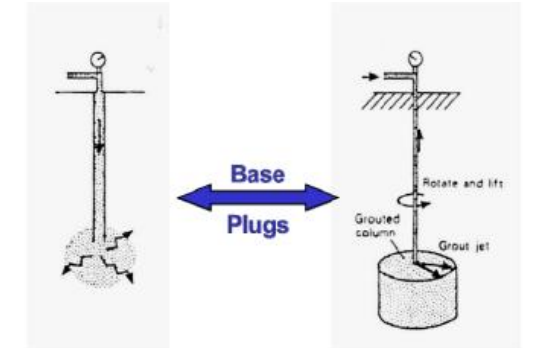
Grouting



Check safety against uplift is necessary

$$F.s. \text{ uplift} = \frac{\delta_{t1} \cdot h_1 + \delta_{t2} \cdot h_2}{\delta_w \cdot H}$$

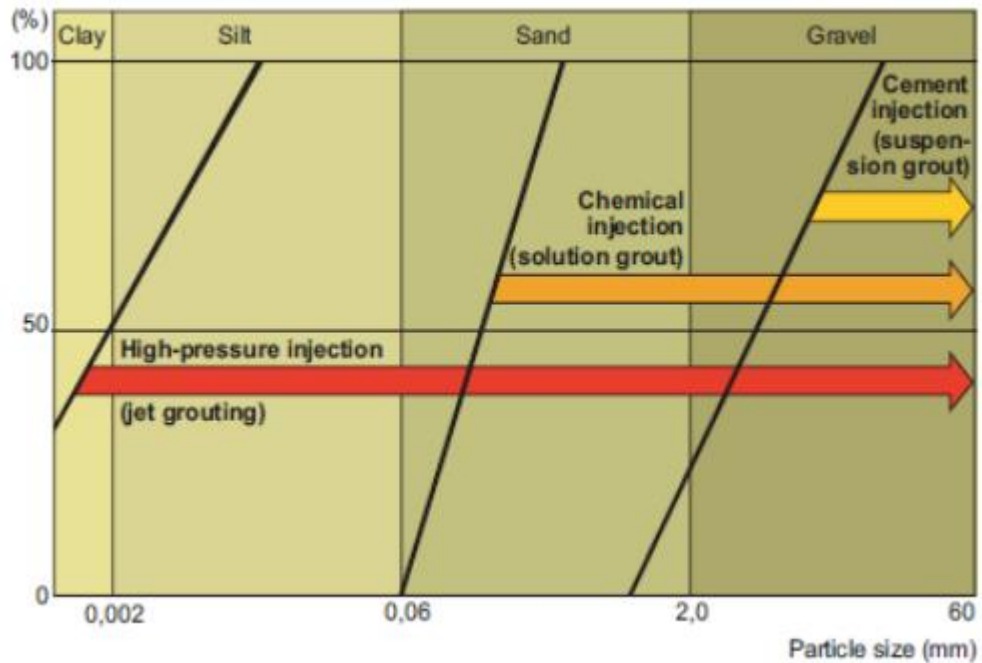
$$\geq 1.2 \quad \text{o.k.}$$



Permeation Grouting
(Penetration/Void Filling)

Jet Grouting
(Replacement or semi-replacement)

Groutability



$$N = (D_{15})_{\text{soil}} / (D_{85})_{\text{grout}} \quad (6.1)$$

Where:

$(D_{15})_{\text{soil}}$

The soil particle size corresponds to accumulative passing of 15%

$(D_{85})_{\text{grout}}$

The grout particle size corresponds to accumulative passing of 85%

$N > 24$

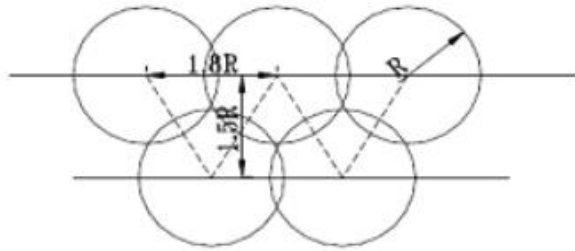
The range for groutable soils using cement grout

$N < 11$

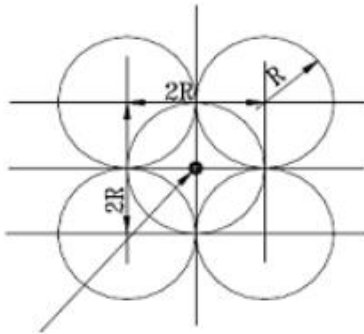
The range for non-groutable soils using cement grout

- Fine content < 12% Groutable
- Fine content < 12% to 20% Groutable with difficulty
- Fine content > 20% Non Groutable

Plug thickness



a- Triangular Pattern.



b- Square Pattern.

Simple Method, $th = \frac{\Delta h}{5}$

As long as the plug material is stable against erosion, its thickness is determined based on the allowable amount of water allowed to seep into the site, which depends on possibility of disposal. The minimum thickness is 1.50 to 2.00m, depending on depth of excavation. Also, the possibility and impact of drop of water level behind the wall should be studied.

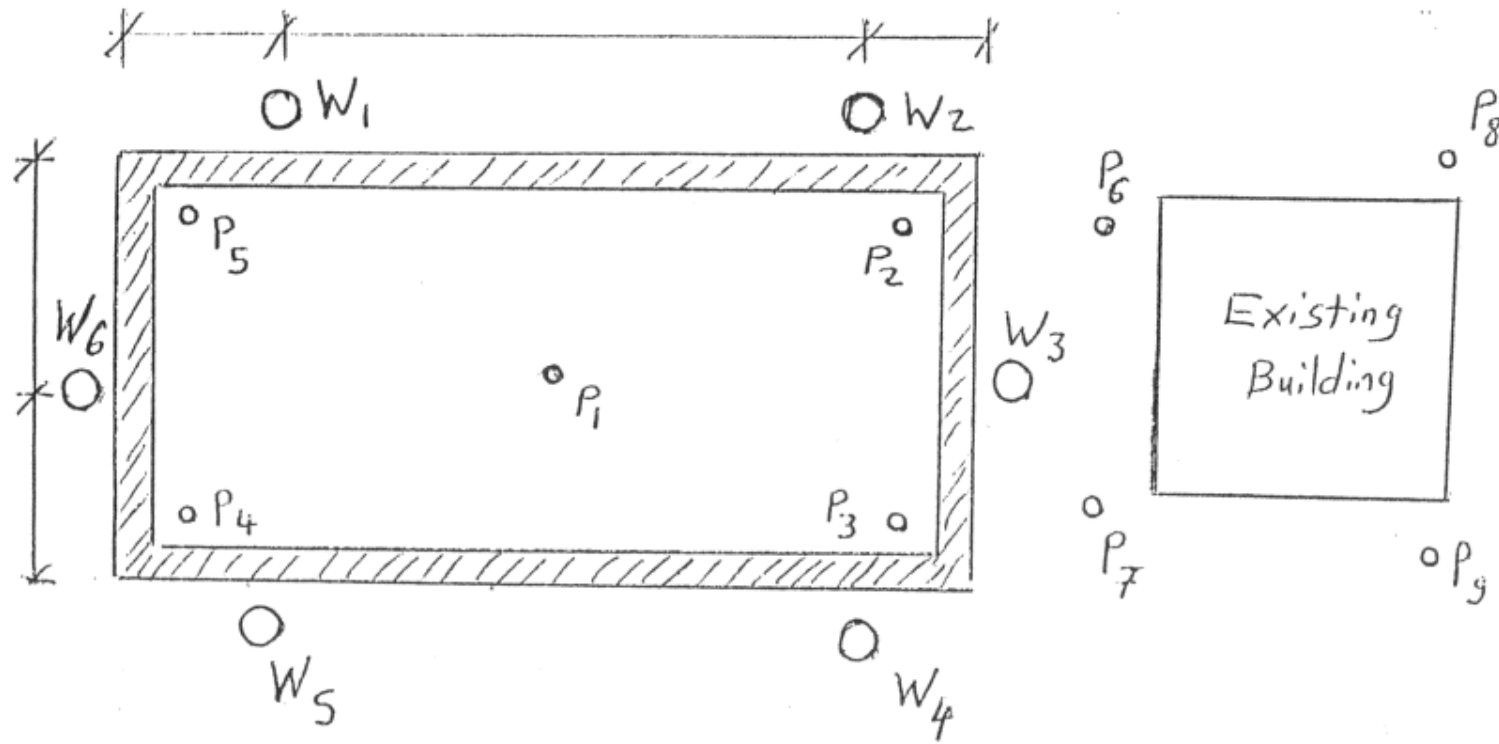
Based on the above criteria, the plug thickness is determined considering the expected permeability of the grouted soil and the hydraulic gradient along the plug thickness. For the detailed method of calculation, refer to (ECP 202-2002 Part 7, Earth Retaining Structures). The amount of water seeping through the grout plug may be estimated as follows:

$$Q = k i A \tag{6.3}$$

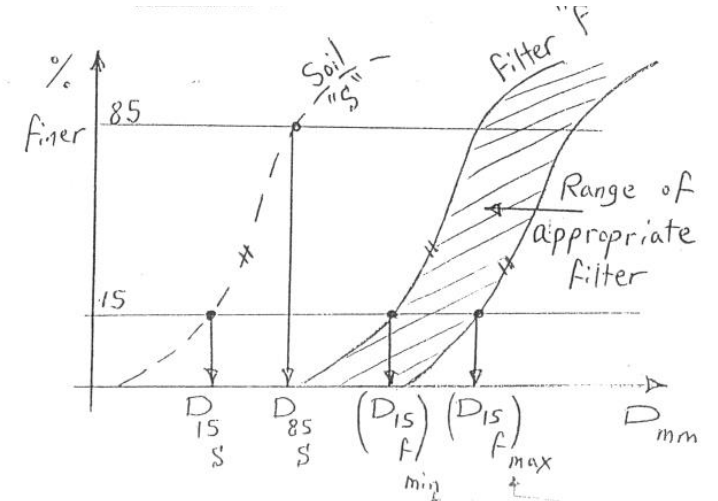
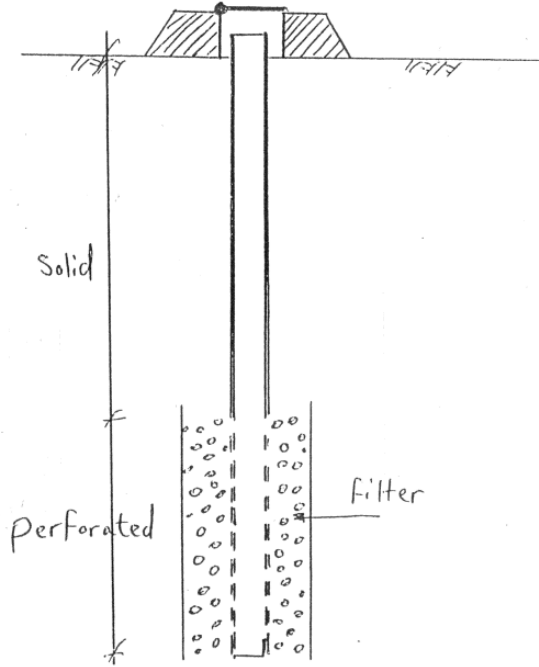
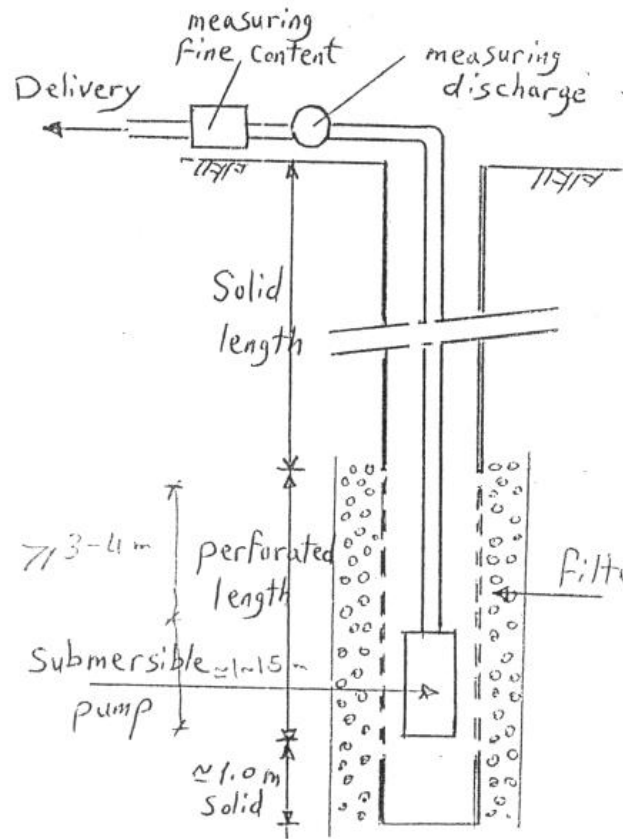
Where:

- Q Amount of water seeping through plug.
- k Plug permeability.
- i Hydraulic gradient along the plug thickness
- A Planar area of the grout plug

Details: Well and Piezometer Locations



Details: Filter



$$\frac{D_{15f}}{D_{15s}} \geq 5 \rightarrow (D_{15f})_{min}$$

$$\frac{D_{15f}}{D_{85s}} \leq 5 \rightarrow (D_{85f})_{max}$$